

APPENDIX 3

Report on Pymble Business Park Transport Assessment of Access Improvements .



Pymble Business Park

Transport Assessment of Access Improvements

Client: Ku-ring-gal Council Reference: JS11530 GTA Consultants Office: Sydney

Issue	Date	Description	Prepared By	Checked By	Approved By
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Introduction



1. Introduction

1.1 Background

Pymble Business Park, located in Sydney's Upper North Shore, comprises a range of land uses including office, business park, freight and logistics, and local light industry, and has been identified in the draft *North Subregional Strategy* (Department of Planning, 2007) as an existing business concentration in the north subregion.

Pymble Business Park is bounded by residential land uses to the north, the Pacific Highway to the east, Ryde Road to the south, and Bullock Park and residential land uses to the west.

Vehicular access is limited to the following two intersections:

- Pacific Highway/Bridge Street (3-way signalised Intersection)
- Ryde Road/West Street (3-way priority intersection left in/left out from West Street only).

During the preparation of the *Ku-ring-gai LEP (Town Centres)* 2010, Pymble Business Park was identified as an area with future growth potential. Ku-ring-gai Council estimates there is growth potential for an additional 31,000m² of office floor space to 2031 as a result of under-utilised sites existing within the Business Park.

It was also identified that the current vehicular access to and from the Business Park was particularly difficult and that any future growth would only exacerbate the issue without capacity improvements or alternative access arrangements being included.

GTA Consultants was commissioned by Ku-ring-gai Council in February 2011 to undertake a traffic assessment of three Council-developed intersection improvement options in the vicinity of Pymble Business Park.

1.2 Purpose of this Report

The objective of the study was to determine whether the proposed intersection improvements developed by Council improved vehicle access to Pymble Business Park both now and in the future. The analysis included a coordinated review of the following three intersections using LinSig Version 3.0 modelling software.

- Pacific Highway/ Ryde Road/ Mona Vale Road (signalised)
- Pacific Highway/ Bridge Street (signalised)
- Ryde Road/ West Street (priority).

The future modelling scenarios also included the addition of development traffic resulting from the estimated future growth in office floor space within the Business Park as provided by Council.

1.3 References

In preparing this Report, reference has been made to the following:

- Ku-ring-gai LEP (Town Centres) 2010
- Ku-ring-gai DCP (Town Centres) 2010
- Pymble Business Park Traffic and Transport Study prepared by Ku-ring-gai Council, November 2009
- Draft Ku-ring-gai Integrated Transport Strategy

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Introduction

- Traffic movement counts and vehicle queue length surveys commissioned by GTA Consultants on 10 February 2011.
- Site visits undertaken during Weekday AM and Weekday PM peak-hours by GTA Consultants on 10 February 2011
- Letter prepared by the RTA to Ku-ring-gai Council regarding the proposed improvement options at Pymble Business Park dated 14 May 2010 (Ref No: 238.5314 Vol 12 SYDog/00861).

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2. Existing Conditions

Pymble Business Park comprises an approximate total Gross Floor Area (GFA) of 76,600m² made up of a combination of office, business park, freight and logistics, and local light industry land uses.

The subject site is bounded by residential land uses to the north, the Pacific Highway to the east, Ryde Road to the south, and Bullock Park and residential land uses to the west. Vehicular access is limited to the following two intersections:

- Pacific Highway/Bridge Street (3-way signalised intersection)
- Ryde Road/West Street (3-way priority intersection left in/left out from West Street only).

The location of the subject site and its surrounding environs is shown in Figure 2.1.

Figure 2.1: Pymble Business Park and Its Environs



Source: Google

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2.1 Road Network

2.1.1 Adjoining Roads

Pacific Highway

Pacific Highway is classified as a State Road by the RTA and is generally aligned in a north-south direction. In the vicinity of the Pymble Business Park, the Pacific Highway is a two-way road configured with 3-lanes each-way and a posted speed limit of 60 km/h.

Ryde Road/Mona Vale Road

Ryde Road/Mona Vale Road is classified as a State Road by the RTA and is generally aligned in an east-west direction. In the vicinity of the Pymble Business Park, Ryde Road/Mona Vale Road is a two-way road configured with 3-lanes each-way and a posted speed limit of 70 km/h.

Ryde Road, just west of West Street, recorded an Annual Average Daily Traffic (AADT) volume of 62,022 vehicles per day in 2005¹.

Bridge Street/West Street/Suakin Street

Bridge Street, West Street and Suakin Street function as local roads within the Pymble Business Park. Bridge Street is aligned in an east-west direction; while West Street and Suakin Street are aligned in a north-south direction. In general, each street is configured with one lane each-way plus kerbside parking on both sides. The posted speed limit on these streets is 50 km/h.

In Bridge Street, West Street and Suakin Street, on-street parking is generally restricted to two hours (2P) between 8:00am and 6:00pm, Monday to Friday.

However in West Street a No Parking restriction applies between 4:00pm and 6:00pm, Monday to Friday along the eastern kerb for the first 40 metres north of the intersection with Ryde Road.

2.1.2 Surrounding Intersections

The following intersections currently exist in the vicinity of the site:

- Pacific Highway/ Ryde Road/ Mona Vale Road (signalised)
- Pacific Highway/ Bridge Street (signalised)
- Ryde Road/ West Street (priority).

2.2 Traffic Volumes

GTA Consultants commissioned traffic movement counts at the three study intersections on Thursday 10 February 2011 during the following peak periods:

- 7:00am and 9:00am
- 4:00pm and 6:00pm.

RTA Station No. 53.021

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Existing Conditions

Following analysis of the survey data, it was identified that the combined network peak of the three study intersections occurred:

- between 7:30am and 8:30am during the Weekday AM peak-hour
- between 4:30pm and 5:30pm during the Weekday PM peak-hour.

The identified Weekday AM and Weekday PM peak-hours are presented in Figure 2.2 and Figure 2.3.



Figure 2.2: Weekday AM Peak-Hour (7:30am - 8:30am)

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Figure 2.3: Weekday PM Peak-Hour (4:30pm - 5:30pm)

For model calibration purposes, vehicle queue length surveys were also undertaken during the same time periods as the traffic movement counts.

2.3 Intersection Operation

To establish the operational performance of the three study intersections during the identified network peak-hours (2011), the following software packages were used:

- LinSig for Pacific Highway/ Ryde Road/ Mona Vale Road and Pacific Highway/ Bridge Street
- SIDRA INTERSECTION for Ryde Road/ West Street.

The results of the analysis are summarised by intersection in the following Sections, with the LinSig and SIDRA INTERSECTION modelling outputs presented in Appendix A and Appendix B respectively.

2.3.1 Pacific Highway/Ryde Road/Mona Vale Road

Table 2.1 presents a summary of the existing operation of the Pacific Highway/ Ryde Road/ Mona Vale Road signalised intersection during the identified network peak-hours.

Peak .	Approach	Demand Flows (pcu)	DOS	Average Delay (sec/pcu)	LOS [1]	Average Maximum Queue (m) [2]
	Pacific Hwy SB (left and through)	808	105.5%	174.6	F	382
	Pacific Hwy SB (through) Lane 3,4	1,571	105.4%	174,6	۴	357
	Pacific Hwy SB (right) Lane 5,6	502	97.6%	153.5	ŕ	97
	Ryde Rd E8 (left, right and through)	. 481	88.1%	45.8	D .	63
	Rydo Rd E8 (right)	· 195	73.1%	80.4	F	. 51
АМ	Ryde Rd E8 (right and U-Tum)	529	88.8% ·	\$4.5	E	76
	Pacific Hwy NB (right)	· 260	106.4%	> 200	F	142
	Pacific Hwy NB (Ihrough) Lane 3,4	497	60.3%	38.0	С	87
	Pacific Hwy N8 (left and through)	592	66,1%	31,2	С	89
	Mana Vale Rd WB (right and U- Turn)	199	80.6%	87,2	· ۴	52
	Mona Vale Rd WB (right, left and through)	348	88.5%	66.5	· E	. 60
	Pacific Hwy SB (left and through)	571	78.2%	42.4	С	117
	Pacific Hwy SB (through) Lane 3,4	996	74.5%	'47,1	С	111
	Pacific Hwy SB (righf) Lone 5,6	650	72,7%	62.2	E	75
	Ryde Rd EB (left, right and through)	596 .	81.6%	37,3	c	104
	Ryde Rd EB (right)	75	36.4%	70.2	Ê	18
М	Ryde Rd EB (right and U-Tum)	453	76.1%	40.6	С	45
	Pacific Hwy NB (right)	419	95.8%	109.5	ŗ	136
	Pocific Hwy NB (through) Lane 3,4	1,213	89,1%	62.7	E	155
	Pacific Hwy N8 (left and through)	888	93.0%	50.5	D	163
	Mona Vale Rd WB (right and U- Turn)	223	80.3%	66.9	Ę	36
,	Mona Vale Rd WB (right, left and through)	252	82.9%	59.6	Ę	, . 37

Table 2.1: 2011 AM/PM Base - Pacific Highway/Ryde Road/Mona Vale Road

(1) Level of Service (LOS) based on the RTA criteria far vehicle delay,

[2] Assumo 6.0m per pcu,

Table 2.1 indicates that:

- During the Weekday AM peak hour the Pacific Highway southbound leg of the intersection experiences significant delays for all movements. Pacific Highway northbound also experiences significant delays for vehicles turning right onto Mona Vale Road.
- Ryde Road eastbound approach and Mona Vale Road westbound approach to the Pacific Highway currently experience moderate delays and queuing during the Weekday AM and PM peak hours.
- During the Weekday PM peak hour the Pacific Highway northbound leg of the intersection experiences significant delays and vehicle queuing for all movements.
- These delays and associated queue lengths were also observed during the site visit and documented within the vehicle queue length surveys.

2.3.2 Pacific Highway/Bridge Street

Table 2.2 presents a summary of the existing operation of the Pacific Highway/Bridge Street signalised intersection during the identified network peak-hours.

Peak	Approach	Demand Flows (pcu)	DOS	Average Delay (sec/pcv)	LOS [1]	Average Maximum Queue (m) [2]
	Pacific Hwy \$8(through) Lane 1,2	1,594	41.9%	1.8	A	3
	Pacific Hwy SB (right and through)	1,463	81.3%	11.3	A	49
	Bridge St (left)	141	26.1%	. 50.8	D	16
AM	Pacific Highway NB (through) Lane 2,3	1,394	53.1%	1.3	A	. 18
	Pacific Highway NB (left and through)	554	45.0%	3.2	A	38
	Pacific Hwy \$8(through) Lane 1,2	1.079	31.7%	1.5	Α	1
	Pacific Hwy SB (right and through)	1,199	66.6%	0.6	٨	19
	Bridge St (left)	294	90.5%	107,8	P.	58
PM	Pacific Highway N8 (through) Lane 2,3	1,740	57.6%	3.7	٨	18
	Pacific Highway N8 (left and through)	· 710	50.9%	3,2	A	78

Table 2.2: 2011 AM/PM Base – Pacific Highway/Bridge Street

[1] Level of Service (LOS) based on the RTA criteria for vehicle delay.
 [2] Assume 6.0m per pcu,

Table 2.2 indicates that:

- During the Weekday AM peak hour the Bridge Street leg of the intersection experiences moderate delays for the left turn only movement onto the Pacific Highway.
- During the Weekday PM peak hour the Bridge Street leg of the intersection experiences significant delays and queuing for the left turn only movement onto the Pacific Highway.
- These delays and associated queue lengths were also confirmed during the site visit.

2.3.3 Ryde Road/West Street

Table 2.3 presents a summary of the existing operation of the Ryde Road/West Street priority intersection during the identified network peak-hours.

Intersection	Peak	leg	Degree of Saturation (DoS)	Average Delay (sec)	95th Percentile Queue (m)	Level of Service (LoS)
		Ryde Road (W)	0.539	8,8	.	A
Ryde	AM	West Street	0.757	55.9	30.9	· D
Road/West Street	PM Ryde Road (W) West Street	Ryde Road (W)	0.524	8.7	-	A
		1.206	243.9	316.0	۴	

Table 2.3: 2011 AM/PM Base - Ryde Road/West Street

Table 2.3 indicates that:

- During the Weekday AM peak-hour, the West Street leg of the intersection experiences moderate delays for vehicles attempting to turn left onto Ryde Road.
- During the Weekday PM peak-hour, the West Street leg of the intersection experiences significant delays and vehicle queuing for the turn left movement onto Ryde Road.
- These delays and associated queue lengths were also confirmed during the site visit.

2.4 Public Transport

A review of the public transport available in the vicinity of the site is summarised in Table 2.4.

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Service	Route No.	Route Description	Location of Stop	Distance to Nearest Stop	Weekday Frequency On/Of Peak	
Bus	560	Pymble to Gordon	Ryde Road near Nadene Place	Minimum 500m	15 mlnutes peak / 1 hour off peak	
B∪s	195	Gordon to St Ives Chase	Gregory and Carr Funerais (850 Pacific Highway)	Minimum 520m	30 minutes peak / 1 hour off peak	
Büs	196	Gordon to Mona Vale	Gregory and Carr Funerals (850 Pacific Highway)	Minimum 520m	30 minutes 6-8am Twice 3:30-5pm 30minutes 7-10:30pm	
Bus	197	Mona Vale to Macquarle University	Ryde Road near West Straet	Minimum 100m	15 mlnutes peak / 30minutes off peak	
Train	n/o	North Shore Line	Pymble Raliway Station	Minimum 650m	3-10 minutes peak / 15 minutes off peak	
Tralņ	n/α	North Shore Line	Gordon Railway Stotion	Minimum 900m	3-10 minutes peak / 15 minutes off peak	

Table 2.4: Public Transport Provision

2.5 Pedestrian Infrastructure

In the vicinity of Pymble Business Park, sealed pedestrian footpaths are located as follows:

- Pacific Highway (both sides)
- Ryde Road (both sides)
- Bridge Street (both sides)
- West Street (both sides)
- ____ Suakin Street (both sides).

Safe crossing points in the vicinity of the Business Park include the following:

- Bridge Street leg of the Pacific Highway/Bridge Street traffic signals
- Eastern and western legs of the Pacific Highway/Ryde Road/Mona Vale Road intersection (combination of marked foot crossings and signalised pedestrian crossings).

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3. Intersection Improvement Options

To improve vehicle access into and out of the Pymble Business Park, both now and in the future, Ku-ring-gai Council developed improvement options for the three study intersections. Each option is detailed in the following sections and presented diagrammatically in Appendix C.

3.1 Pacific Highway/Ryde Road/Mona Vale Road

For the Pacific Highway/Ryde Road/Mona Vale Road signalised intersection, the following improvements are proposed:

- Extension of the left-turn slip lane storage on the Ryde Road off-ramp to 140 metres
- Extension of the through/right-turn lane on the Ryde Road off-ramp to 110 metres.

The proposed improvements are presented in Figure 3.1 below.





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Intersection Improvement Options

3.2 Pacific Highway/Bridge Street

For the Pacific Highway/Bridge Street signalised intersection, the following improvements are proposed:

- Addition of a 25 metre left-turn bay on the Pacific Highway (S) leg of the Intersection
- Addition of a signal-controlled pedestrian crossing across the Pacific Highway (S) leg of the intersection.

The proposed improvements are presented in Figure 3.2 below.

Figure 3.2: Intersection Improvements – Pacific Highway/Bridge Street



3.3 Ryde Road/West Street

For the Ryde Road/West Street priority intersection, the following improvements are proposed:

- Installation of new traffic signals to control vehicular access into and out of West Street
- Addition of a second left-turn only lane on the West Street leg
- Addition of a signal-controlled pedestrian crossing across the West Street leg of the intersection.

The proposed improvements are presented in Figure 3.3.

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Intersection Improvement Options



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Traffic impact Assessment



4. Traffic Impact Assessment

4.1 Future Growth Estimate

Ku-ring-gai Council have estimated that there is growth potential for an additional 31,000m² of office floor space up to 2031 which was based on the findings of the Ku-ring-gai and Hornsby Subregional Employment Lands Study (KHSELS), and consideration of current under-utilised sites within Pymble Business Park.

To present a robust analysis of the proposed access improvements, it has been assumed for modelling purposes that the additional 31,000m² of office floor space would be delivered by 2021.

4.2 Traffic Generation

For business parks, the RTA's Guide to Traffic Generating Developments (Version 2.2, October 2002) recommends the following trip generation rates:

Peak Vehicle Trips (PVT) = 1.2 vehicle trips / hour / 100m² of GLFA⁽²⁾

where 'GLFA' represents the Gross Leasable Floor Area

Assuming that the GLFA represents 80% of the Gross Floor Area (GFA), application of the RTA rates to the full growth potential of Pymble Business Park (31,000m²) would generate an additional 298 vehicle trips during the Weekday AM and PM peak-hours.

4.3 Trip Distribution and Assignment

Council's *Pymble Business Park Traffic and Transport Study* indicates that during the Weekday AM peak (0730-0830), 75% of development traffic would be inbound and 25% of development traffic would be outbound.

Conversely during the Weekday PM peak (1630-1730), 25% of development traffic would be inbound and 75% of development traffic would be outbound.

Application of these in/out percentage splits to the estimated development traffic would result in:

- Weekday AM Peak 224 vehicles inbound, 74 vehicles outbound
- Weekday PM Peak 74 vehicles inbound, 224 vehicles outbound.

Future development traffic associated with Pymble Business Park has been assigned across the wider road network using the following assumptions:

- 40% to/from the north via Pacific Highway
- 20% to/from east via Mona Vale Road
- 20% to/from south via Pacific Highway
- 20% to/from west via Ryde Road.

These percentage splits generally correlate with the findings of the *Draft Ku-ring-gai Integrated Transport* Strategy which determined that the majority of work trips to the Ku-ring-gai Local Government Area (LGA)

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^{*} Trip rate and GFA to GLFA conversion factor extracted from Pymble Business Park Traffic and Transport Study prepared by Ku-ting-gai Council, November 2009

originate from either within the Ku-ring-gai LGA or from the Hornsby LGA. Hence the greater percentage of development trips assigned to/from the north.

A summary of the trip distribution and assignment of development traffic is presented diagrammatically in Figure 4.2 and Figure 4.2.



Figure 4.1: Development Traffic Distribution and Assignment – Weekday AM Peak

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Traffic Impact Assessment



Figure 4.2: Development Traffic Distribution and Assignment – Weekday PM Peak

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Modelling Methodology



5. Modelling Methodology

5.1 Introduction

The modelling analysis included an assessment of the following three (3) study intersections:

- Pacific Highway/ Ryde Road/ Mona Vale Road (4-way signalised intersection)
- Pacific Highway/ Bridge Street (3-way signalised intersection)
- Ryde Road/ West Street (3-way priority intersection left in/ left out from West Street only).

Intersection capacity assessments were undertaken for the Weekday AM (7:30-8:30) and Weekday PM (4:30-5:30) peak-hours for the following five (5) scenarios presented in Table 5:1 below.

Table 5.1: Modelling Scenarios

Tille	Description
Existing	2011 Base (presented in Section 2.3)
Scenario 1	2021 8ase + 8ackground Growth + Development ('Do Nothing' scenario)
Scenario 2	2021 Base + Background Growth + Development + Intersection Improvements
Scenario 3	2021 Base + Background Growth + Davelopment + Intersection Improvements + Right-Turn Phase at Bridge Street
Scenario 4	2021 Base + Background Growth + Development + Intersection Improvements + Ryde Road (EB) Off- Ramp Improvements

It should be noted that from Scenario 2 to Scenario 4, the intersection of Ryde Road/West Street is modelled as a signal-controlled intersection with two left turn lanes on the West Street leg (as per Figure 3.3).

Furthermore for Scenario 3, with the addition of a right-turn from Bridge Street onto Pacific Highway (S), there was a need to reassign both growthed base traffic flows and future development traffic associated with the increased office floor space. For simplicity, the following assumptions were adopted:

- Growthed base traffic flows recorded for the left-turn movement from West Street onto Ryde Road (E) were split 50/50 between the left-turn movement at West Street and the modelled rightturn movement at Bridge Street onto Pacific Highway (S).
- Future development traffic assigned to the left-turn movement from West Street onto Ryde Road
 (E) (Future Weekday AM = 44 veh/hr; Future Weekday PM = 134 veh/hr) were also split 50/50
 between the left-turn movement at West Street and the modelled right-turn movement at Bridge
 Street onto Pacific Highway (S).

These modelling assumptions were considered to be reasonable as 50% of businesses would be accessed via West Street/Suakin Street and 50% of businesses would be accessed via Bridge Street.

5.2 Modelling Packages

5.2.1 LinSig

LinSig is a computer software package for the assessment and design of traffic signal intersections either individually or as a network comprised of multiple intersections. It is generally used to construct a model of the intersection or network which can then be used to assess different designs and methods of operation. Apart from stand-alone intersections, it is used for multiple traffic signal intersections, complex networks, signalled roundabouts, and road networks which may include traffic signal pedestrian crossings and priority intersections as well as traffic signal intersections.

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Modelling Methodology

For the purpose of the LinSig analysis, the following assumptions have been made:

- Given that traffic flows in LinSig are presented in Passenger Car Units (PCU), the following conversion was adopted for the existing traffic volumes:
 - Car = 1 PCU
 - Heavy Goods Vehicle = 2.3 PCUs
- Saturation flow values of 1,800 pcus/h have been adopted for through and turning lanes. Generally, saturation flow should be measured on site for each lane or group of lanes for each peak period. Given that no site measurements have been undertaken as part of this project, basic saturation flows have been based on the geometric settings of a typical lane in the study area.
- Given that saturation flow is dependent on a number of factors including the site conditions and the proportion of heavy vehicles, it is expected that the values adopted for the analysis are both conservative and consistent with the Austroads classifications for lane saturation flows.
- Lane lengths have been based on the existing intersection layout with short lanes used to represent how road space is currently used.
- Phase sequence arrangements, durations and cycle times for the base models have been based on the Intersection Diagnostic Monitor (IDM) data provided by RTA.
- A start lag of 5 seconds has been applied to vehicle movements that run simultaneously with pedestrian movements (i. e. left and right turning movements) to represent the delay to vehicles caused by pedestrians.

It should be noted that the 2011 Base scenario, the Pacific Highway/ Ryde Road/ Mona Vale Road and Pacific Highway/ Bridge Street signalised intersections were calibrated in LinSig using IDM data collected by the RTA.

For all future scenarios (Scenario 1 through to Scenario 4) however, all signalised intersections were optimised within LinSig.

5.2.2 SIDRA INTERSECTION

The operation of the Ryde Road/West Street priority intersection was assessed using SIDRA INTERSECTION³, a computer based modelling package which calculates intersection performance, in the 2011 Base and Scenario 1.

The commonly used measure of intersection performance, as defined by the RTA, is vehicle delay. SIDRA INTERSECTION determines the average delay that vehicles encounter and provides a measure of the level of service.

Table 5.2 shows the criteria that SIDRA INTERSECTION adopts in assessing the level of service.

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Modelling Methodology

Table 5.2: SIDRA INTERSECTION Level of Service Criteria

Level of Service (LOS)	Average Delay per vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Sign	
A	Less than 14	Good operation	Good operation	
8	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity	
с	29 to 42	Satisfactory	Satisfactory, but accident study required	
D	43 to 56	Near capacity	Near capacity, accident study required	
E	57 lo 70	At capacity, at signals incidents will cause excessive delays	At capacity, requires other control mode	
F	Greater than 70	Extra capacily required	Extreme delay, major treatment required	

5.3 Model Calibration

Each study intersection was calibrated for the 2011 Base scenario using a combination of the following data sources:

- Vehicle queue length surveys
- Recent turn count data
- IDM data provided by the RTA for the z existing signalised intersections.
- Observations of traffic behaviour and vehicle queuing recorded during site visits.

Where traffic behaviour and vehicle queuing observations recorded during the site visit did not correlate with the vehicle queue length surveys, the vehicle queue length surveys were used as the primary source for base model calibration.

5.4 Future Traffic Growth

To establish the future traffic volumes associated with the 2021 modelling scenarios, annual growth factors were applied to the 2011 turn count data. The following annual growth fates, as provided by the RTA, were applied:

- Pacific Highway 0.6% per annum
- Ryde Road/ Mona Vale Road 0.8% per annum.

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6. Modelling Results

6.1 Scenario 1

Scenario 1 represents a 'Do Nothing' option whereby existing 2011 AM/PM peak-hour flows are growthed to 2021 traffic flows (using the RTA factors presented in Section 5.4). Development traffic associated with the future growth of the Pymble Business Park is assigned to the study intersections, and no improvement options included (i.e. the road geometry of the study intersections remains as existing).

For Scenario 1, the following software packages were used:

- LinSig for Pacific Highway/ Ryde Road/ Mona Vale Road and Pacific Highway/ Bridge Street
- SIDRA INTERSECTION for Ryde Road/ West Street.

The results of the analysis are summarised by intersection in the following Sections, with the LinSig and SIDRA INTERSECTION modelling outputs presented in Appendix A and Appendix B respectively.

6.1.1 Pacific Highway/Ryde Road/Mona Vale Road

Table 6.1 presents a summary of the future operation of the Pacific Highway/ Ryde Road/ Mona Vale Road signalised intersection during the identified network peak-hours.

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Peak	Approach	Demand Flows (pcu)	DOS	Average Delay (sec/pcv)	LOS (1]	Average Maximum Queue (m) [2]
	Pacific Hwy SB (left and through)	902	113.3%	>200	٦	779
	Pacific Hwy SB (through) Lane 3,4	1,738	113.0%	>200	F	562
	Pacific Hwy SB (right) Lane 5,6	566	100.1%	166.6	۶.	117
	Ryde Rd EB (left, right and through)	562	109.3%	>200	, F	262
	Ryde Rd E8 (right)	247	107.2%	>200	F	140
AM	Ryde Rd EB (right and U-Turn)	591	109.8%	>200	۶	251
7.04	Pacific Hwy NB (right)	304	112.6%	>200	۴,	203 .
	Pacific Hwy NB (Ihrough) Lane 3,4	1,024	66.8%	38,9	Ċ	102
	Pacific Hwy NB (left and through)	682	72.6%	31,5	С	103
	Mona Vale Rd W8 (right and U- Turn)	265	114,4%	``>200	F	189
	Mona Vale Rd WB (right, left and through)	432	114.8%	>200	'n	273
	Pacific Hwy S8 (left and thraugh)	590	75.6%	38.4	С	116
	Pacific Hwy SB (through) Lane 3,4	1,028	,71.9%	42.6	·D	111
	Pacific Hwy SB (right) Lane 5.6	664	66.8%	55.2	E	75
	Ryde Rd E8 (left, right and through)	. 641	. 89.9%	50.6	D	123
	Ryde Rd EB (right)	78	53,6%	87.6	ſ	21
РМ	· Ryde Rd E8 (right and U-Turn)	455	94.6%	76.0	F	69
1 101	Pacific Hwy NB (right)	434	88.8%	78.2	۴	118
	Pacific Hwy N8 (through) Lane 3,4	1,247	85.1%	53.3	D	148
	Pacific Hwy N8 (left and through)	896	90.0%	42.3	С	150
	Mona Vale Rd WB (right and U- Turn)	225	87.4%	82,0	F	- 39
	Mono Vale Rd WB (right, left and through)	247	88.6%	74.3	. F	40

Table 6.1: Scenario 1 - Pacific Highway/Ryde Road/Mona Vale Road (Optimised Results)

Level of Service (LOS) based on the RTA criteria for vehicle delay.
 Assume 6.0m per pcu.

Table 6.1 indicates that:

- During the Weekday AM peak hour, with the inclusion of background growth on the Pacific Highway and Ryde Road plus development traffic generated by the future expansion of the Pymble Business Park, all approaches to the Intersection with the exception of Pacific Highway northbound through and left movements, experience significant delays and queuing.
- During the Weekday AM peak hour, these legs would exceed operational capacity under the current intersection layout.
- During the Weekday PM peak hour, the majority of approaches to the intersection experience delay and queuing, in particular Ryde Road and Mona Vale Road approaches.

6.1.2 Pacific Highway/Bridge Street

Table 6.2 presents a summary of the future operation of the Pacific Highway/Bridge Street signalised intersection during the identified network peak-hours.

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Peak	Approach	Demand Flows (pcu)	DØ\$	Average Delay (sec/pcu)	LOS [1]	Average • Maximum Queue (m) [2]
	Pacific Hwy SB(through) Lane 1,2	1,774	50.1%	2.0	• A	3
	Pacific Hwy SB (right and through)	1,700	94.4%	24.2	8	103
AM .	Bridge St (left)	180	33,1%	51,9	D	20
	Pacific Highway N8 (thraugh) Lane 2,3	1,627	59.9%	5.2	Α.	29
	Pacific Highway N8 (left and through)	649	51.6%	4.3	A	51
	Pacific Hwy SB(through) Lane 1,2	1,117	32.8%	1.5	٨	
	Pacific Hwy SB (right and through)	1,226	68.1%	5.7	A	19
	Bridge St (left)	293	60.2%	52.8	E	38
PM	Pacific Highway NB (through) Lane 2,3	1,767	63 <i>.</i> 2%	5,5	A	40
	Pacific Highway NB (left and through)	733	56.6%	4.7	A	78

[1] Level of Service (LOS) based on the RTA criteria for vehicle delay. (2] Assume 6.0m per pou,

Table 6.2 indicates that:

- During the Weekday AM peak hour the Bridge Street leg of the intersection experiences moderate delays for the left turn only movement onto the Pacific Highway.
- The Pacific Highway experiences queuing during the AM (southbound) and PM (northbound) peak hours.
- During the Weekday PM peak hour the Bridge Street leg of the intersection experiences significant delays and queuing for the left turn only movement onto the Pacific Highway.

6.1.3 Ryde Road/West Street

PM

Table 6.3 presents a summary of the future operation of the Ryde Road/West Street priority intersection during the identified network peak-hours.

Intersection	feak	Leg	Degree of Saturation (DoS)	Average Delay (sec)	95lh Percentile Queve (m)	
	1	Ryde Road (W)	0.586	8.8	*	T
Ryde	AM	West Street	1.247	301.7	197.7	<u> </u>
Road/West					1	1

Ryde Road (W)

West Street

Table & 3: Scenario 1 - Ryde Road/West Street

Table 6.3 indicates that during both the Weekday AM and Weekday PM peak hour, the Ryde Road/West Street priority intersection experiences unacceptable delays and vehicle queuing exceeding the optimal capacity of the intersection.

0.569

1.865

8.8

> 360.0

> 500.0

6.2 Scenario 2

Street

Scenario 2 is similar to Scenario 1 but with the inclusion of proposed intersection improvements at the Pacific Highway/Bridge Street intersection (see Section 3.2) and the Ryde Road/West Street (see Section 3.3).

Level of Service (LoS) A

A

F



As the Ryde Road/West Street intersection is signalised in Scenario 3, the LinSig software packages was used for all study intersections.

The results of the analysis are summarised by intersection in the following Sections, with the LinSig and modelling outputs presented in Appendix A.

6.2.1 Pacific Highway/Ryde Road/Mona Vale Road

Table 6.4 presents a summary of the future operation of the Pacific Highway/Ryde Road/Mona Vale Road signalised intersection during the identified network peak-hours.

Peak	Approach	Demand Flows (pcu)	DOS	Average Delay (sec/pcv)	LOS [1]	Average Maximum Queve (m) [2]
	Pacific Hwy SB (left and through)	907	113.9%	>200	F	798
	Pacific Hwy SB (through) Lane 3,4	1,733	113.7%	· >200	F	579
	Pacific Hwy SB (right) Lane 5.6	566	96.7%	134.7	F	101
	Ryde Rd EB (left, right and librough)	561	108.8%	>200	F	258
	Ryde Rd E8 (right)	248	107.6%	>200	٦	142
АМ	Ryde Rd E8 (right and U-Turn)	589	109.4%	>200	F	244
710	Pacific Hwy NB (right)	· 304	112.6%	>200	F	203
	Pocific Hwy NB (through) Lone 3,4	1.026	67.1%	38.7	С	103
	Pacific Hwy NB (left and through)	680	72.3%	31,4	С	103
	Mona Vale Rd WB (right and U- Turn)	264	114.0%	>200	F	185
	Mona Vale Rd W8 (right, left and through)	432	114.8%	>200 -	F	273
	Pacific Hwy SB (left and through)	613	84,0%	45.8	D	135
	Pacific Hwy SB (through) Lane 3.4	1,047	81.4%	47.8	D	129
	Pacific Hwy SB (right) Lano 5,6	688	72.7%	56.5	Ē	81
	Ryde Rd EB (left, right and through)	606	91.3%	50.9	D	142
•••	Ryde Rd E8 (right)	47	75.8%	81.7	F	42
РМ	Ryde Rd EB (right and U-Tum)	547	93.8%	61.9	E	76 ·
1.791	Pacific Hwy N8 (right)	445	96.1%	107.9	F	144
	Pacille Hwy N3 (Ihrough) Lone 3,4	1,297	· 95.2%	80.8	F	189
	Pacific Hwy NB (left and through)	934	97.3%	67.4	E	207
	Mono Vale Rd WB (right and U- Tum)	250	97.5%	133,6	F	73
٨	Mono Vale Rd WB (right, left and - through)	279	98.2%	123,4	F	77

Table 6.4: Scenario 2 – Pacific Highway/Ryde Road/Mona Vale Road (Oplimised Results)

(1) Level of Service (LOS) based on the RTA criteria for vehicle delay.

[2] Assume 6.0m per pou,

Table 6.4 indicates that:

- During the Weekday AM peak hour, with the addition of background and development traffic combined with the proposed intersection improvement works, all approaches to the intersection with the exception of Pacific Highway northbound through and left movements, experience significant delays and vehicle queuing.
- During the Weekday AM peak hour, these legs would exceed operational capacity under the current intersection layout.

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 During the Weekday PM peak hour, the majority of approaches to the intersection experience delay and queuing, in particular Ryde Road and Mona Vale Road approaches.

6.2.2 Pacific Highway/Bridge Street

Table 6.5 presents a summary of the future operation of the Pacific Highway/Bridge Street signalised intersection during the identified network peak-hours.

Peak	Approach	Demand Flows (pcv)	DOS	Average Delay (sec/pcu)	LOS [7]	Average Maximum Queue (m) [2]
	Pacific Hwy SB(through) Lane 1,2	1,784	58.3%	5.3	A	62
	Pacific Hwy SB (right and through)	1,690	103.5%	108.7	F	671
	Bridge St (left)	180	29,3%	45.4	D	19
AM	Pacific Highway N8 (through) Lane 2,3	1,510	61.7%	6,1	A	30
	Pacific Highway N8 (left and through)	765	59.5% ·	6.0	Ą	85
	Pacific Hwy SB(through) Lana 1,2	1,168	39.4%	3.8	А	31
	Pacific Hwy SB (right and through)	1,331	82,7%	16.4	В	128
PM	Bridge St (left)	383	68.0%	58.9	·· E	48
	Pacific-Highway N8 (through) Lane 2,3	1,796	69.3%	7.1	A	34
	Pacific Highway NB (loft and through)	825	66.0%	7.4	A	122

Table 6.5: Scenario 2 - Pacific Highway/Bridge Streef (Optimised Results)

[1] Level of Service (LOS) based on the RTA criterio for vehicle delay.

[2] Assume 6.0m per pcu,

Table 6.5 indicates that:

- During the Weekday AM peak hour the Bridge Street leg of the intersection experiences significant delays for the left turn only movement onto the Pacific Highway.
- The Pacific Highway experiences queuing for the southbound movement during the AM peak hour and exceeds operational capacity under the Scenario 3 intersection layout.
- During the Weekday PM peak hour the Bridge Street leg of the intersection experiences significant delays and queuing for the left turn only movement onto the Pacific Highway.

6.2.3 Ryde Road/West Street

Table 6.6 presents a summary of the future operation of the Ryde Road/West Street priority intersection during the identified network peak-hours.

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Peak	Approach	Demand Flows (pcv)	DOS	Average Delay (sec/pcu)	LOS [1]	Average Maximum Queue (m) [2]
	Rydo Rd E8 (left and through) Lane	1.019	72.3%	10.5	'A	115
AM	Ryde Rd EB (through) Lane 2:3	2,311	74.0%	9.3	A	124
	West St (left) Lane 1,2	184	69,7%	88.6	F	32
	Ryde Rd EB (left and through) Lane	968	82,5%	25.0	В	178
PM	Ryde Rd E8 (Ihrough) Lane 2,3	2,229	85.2%	24.6	3	210
	West St (left) Lane 1,2	525	92.1%	75.6	F	110

Table 6.6 indicates that:

During the Weekday AM and PM peak hour the West Street leg of the intersection experiences significant delays and moderate queuing for the left turn only movement onto Ryde Road.

During the Weekday PM peak hour the Ryde Road leg of the intersection experiences reasonable delays and queuing for the eastbound approach to the intersection.

6.3 Scenario 3

Scenario 3 is similar to Scenario 2 but with the addition of a right-turn phase from Bridge Street onto Pacific Highway (S).

The results of the analysis are summarised by intersection in the following Sections, with the LinSig and modelling outputs presented in Appendix A.

Pacific Highway/Ryde Road/Mona Vale Road 6.3.1

Table 6.7 presents a summary of the future operation of the Pacific Highway/ Ryde Road/ Mona Vale Road signalised intersection during the identified network peak-hours.

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Peak	Approach	Demand Flows (pcu)	DOS	Average Delay (sec/pcu)	LOS [1]	Average Maximum Queve (m) [2]
	Pacific Hwy SB (left and through)	918	112.8%	>200	F	783
	Pacific Hwy SB (through) Lane 3,4	1,777	112.6%	>200	F	559
	Pacific Hwy SB (right) Lane 5,6	594	102.3%	186.0	F	131
	Ryde Rd E8 (left, right and through)	560	i09.2%	>200	F	258
	Ryde Rd EB (right)	233	106.7%	>200	F	131
M	Ryde Rd E8 (right and U-Turn)	540	109.5%	>200	F	230
	Pacitic Hwy N8 (right)	304	112.6%	>200	3	203
•	Pacific Hwy N8 (through) Lane 3,4	1,016	66.4%	37.6	С	102
	Pacific Hwy N8 (left and through)	687	72.4%	31,0	С	104
	Mona Vale Rd WB (right and U- Turn)	263	113.5%	>200	f	181
	Mọna Vale Rd W8 (right, left and through)	433	115.4%	>200	F	189
	Pacific Hwy S8 (left and through)	669	98.7%	87.6	F	207
	Pacific Hwy SB (Ihrough) Lane 3.4	1,157	97,6%	82.7	F	190
	Pacific Hwy SB (right) Lane 5,6	769	· 74.4%	60.1	E	96
	Ryde Rd E8 (left, right and through)	576	90,1%	43.7	D	140
	Ryde Rd EB (right)	. 111	57.2%	74,1	F	28
м	Ryde Rd EB (right and U-Turn)	427	84.5%	51.6	D	52
	Pacific Hwy N8 (right)	445	88,7%	76.8	۴	121
	Pacific Hwy NB (through) Lane 3,4	1,277	105.7%	197.1	ŗ	310
·····	Pacific Hwy NB (left and through)	937	106.8%	178.7	F	388
	Mona Vale Rd WB (right and U- Turn)	250	84.9%	76.0	f	48
	Mona Vale Rd WB (right, left and through)	277	85.9%	65.2	E	48

able & 7. Scenario 3	- Pacific Water (Rúda	Road/Mona Vale Road (Optimised Results)	

[1] Level of Service (LOS) based on the RTA afterta for vehicle delay,

(2) Assume 6.0m per peu,

Table 6.7 indicates that:

- During the Weekday AM peak hour, with the addition of background and development traffic combined with the proposed intersection improvement works, all approaches to the intersection with the exception of Pacific Highway northbound through and left movements, experience significant delays and queuing.
- The Pacific Highway southbound movement during the AM peak hour experiences extensive gueues and delay on approach to the intersection.
- During the Weekday AM peak hour, these legs would significantly exceed operational capacity under the current intersection layout.
- During the Weekday PM peak hour, the majority of approaches to the intersection experience delay and gueuing, in particular Ryde Road and Mona Vale Road approaches.

6.3.2 Pacific Highway/Bridge Street

Table 6.8 presents a summary of the future operation of the Pacific Highway/Bridge Street signalised intersection during the identified network peak-hours.

Peak	Approach	Demand Flows (pcu)	DOS	Average Delay (sec/pcu)	LOS [1]	Averaga Maximum Queue (m) [2]
	Pacific Hwy SB(Ihrough) Lane 1,2	1,798	58.9%	5.3	A	63
	Pacific Hwy.SB (right and through)	1,669	102.1%	40.4	С	610
	8ridge St (left and right)	270.	119.2%	>200	Ê	185
ам	Pacific Highway NB (through) Lane 2,3	1,496	64.9%	8.9	A	12]
	Paclfic Highway NB (left and through)	780	65.1%	7.8	A	90
	Pacific Hwy SB(through) Lane 1,2	1,230	50.4%	10,9	A	70
	Pacific Hwy SB (right and through)	1,194	89.4%	28,3	ß	230
PM	Bridge St (left)	642	110.6%	>200	F	348
	Pacific Highway NB (through) Lane 2.3	1,775	109.4%	>200	F	466
	Pacific Highway N8 (left and through)	848	109.3%	>200	F	442

Table 6.8: Scenario 3 - Pacific Highway/Bridge Street (Optimised Rasults)

Level of Service (LOS) based on the RTA critoria for vehicle delay.
 Assume 6.0m per pau,

Table 6.8 indicates that:

• During the Weekday AM and PM peak hour the Bridge Street leg of the intersection experiences

- significant delays and queuing for the left and right turns onto the Pacific Highway.
 The Pacific Highway experiences significant queuing during the AM peak hour (southbound) and
 DM workshows (as the part of a part of a part is a part of a part of a part is a part is a part is a part of a
 - PM peak hour (northbound) and exceeds operational capacity under the Scenario 4 intersection layout.

6.3.3 Ryde Road/West Street

Table 6.9 presents a summary of the future operation of the Ryde Road/West Street priority intersection during the identified network peak-hours.

Peak	Approach	Demand. Flows (pcu)	DOS	Average Delay (sec/pcu)	LOS [1]	Average Maximum Queue (m) [2]
,	Ryde Rd EB (left and through) Lane	1.027	69.8%	7.9	A	94
AM	Ryde Rd E8 (through) Lane 2,3	2,299	70.6%	6.7	, A	96
	West St (left) Lane 1,2	92	58.7%	99.0	۴	18
	Ryde Rd E8 (left and through) Lane	956	, 71.6%	12.9	٨	121
РМ	Ryde Rd EB (through) Lane 2.3	2,218	74,8%	12.2	٨	. 140
	West St (left) Lane 1,2	260	81.4%	80.9	E	. 55

Table 6.9; Scenario 3 - Ryde Road/West Street (Optimised Results)

Table 6.9 indicates that:

- During the Weekday AM and PM peak hour the West Street leg of the intersection experiences significant delays for the left turn only movement onto Ryde Road.
- During the Weekday PM peak hour the Ryde Road leg of the intersection experiences reasonable delays and queuing for the eastbound approach to the intersection.

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6.4 Scenario 4

Scenario 4 is similar to Scenario 2 but with the inclusion of proposed intersection improvements at the Pacific Highway/ Ryde Road/ Mona Vale Road intersection (see Section 3.1).

The results of the analysis are summarised by intersection in the following Sections, with the LinSig and modelling outputs presented in Appendix A.

6.4.1 Pacific Highway/Ryde Road/Mona Vale Road

Table 6.10 presents a summary of the future operation of the Pacific Highway/Ryde Road/Mona Vale Road signalised intersection during the Identified network peak-hours.

Peak	Approach	Demand Flows (pcv)	200	Average Delay (sec/pcu)	· LOS [1]	Average Maximum Quoue (m) [2]
	Pacific Hwy SB (left and through)	903	111.6%	>200	F	744
	Pacific Hwy SB (through) Lane 3,4	1,737	111.3%	>200	f	525
	Pacifle Hwy SB (right) Lane 5,6	566	95.7%	130.5	F	100
	Ryde Rd EB (left, right and through)	641	103.8%	162,8	F	191
	Ryde Rd EB (right)	207	106.7%	>200	f	118
AM	Ryde Rd EB (right and U-Tum)	551	109.8%	>200	4	229
7.941	Pacific Hwy N8 (right)	304	107,5%	>200	F	169
	Pacific Hwy NB (through) Lane 3.4	1,027	66.0% .	37,8	С	101
	Pacific Hwy NB (left and through)	· 679	71.2%	30.5	С	101
	Mona Vale Rd W8 (right and U- Turn)	265	108.6%	>200	F	154
	Mona Vale Rd WB (right, left and through)	431	110.5%	`>200	÷	228
	Pacific Hwy S8 (left and through)	602	83.8%	47.6	D	/ 132
	Pacific Hwy S8 (through) Lane 3.4	1,058	81.1%	51.2	D	126
	Pacific Hwy \$8 (right) Lane 5,6	688	72.7%	59.4	E	81
	Ryde Rd E8 (leit, right and through)	598	90.7%	48.4	D	145
	Ryde Rd E8 (right)	136	59.0%	60,4	E	33
PM	Ryde Rd EB (right and U-Turn)	565	86.8%	42.5	С	63
1 141	Pacific Hwy N8 (right)	445	96.1%	107,9	۴	144
	Pacific Hwy NB (Ihrough) Lane 3,4	1,297	97.1%	91.4	۶	201
	Pacific Hwy NB (left and through)	935	98.8%	77.9	۴	228
	Mona Vale Rd WB (right and U- Turn)	250	108,2%	, >200	F	130
	Mona Vale Rd WB (right, left and through)	279 .	107.9%	>200	F	130

Table 6.10: Scenario 4 - Pacific Highway/Ryde Road/Mona Vale Road (Optimised Results)

[1] Level of Service (LOS) based on the RIA criteria for vehicle delay.

[2] Assume 6.0m per pcu,

Table 6.10 indicates that:

 During the Weekday AM peak hour, with the addition of background and development traffic combined with the proposed intersection improvement works for Scenario 5, all approaches to the intersection with the exception of Pacific Highway northbound through and left movements, experience significant delays and queuing.

- The Pacific Highway southbound movement during the AM peak hour experiences extensive gueues and delay on approach to the intersection.
- During the Weekday AM peak hour, these legs would exceed operational capacity under the Scenario 5 intersection layout.
- During the Weekday PM peak hour, the majority of approaches to the intersection experience delay and queuing.

6.4.2 Pacific Highway/Bridge Street

Table 6.11 presents a summary of the future operation of the Pacific Highway/Bridge Street signalised intersection during the identified network peak-hours.

Peak	Approach	Demand Flows (pcu)	DQS	Average Delay (sec/pcu)	LOS (1)	Average Maximum Queue (m) [2]
	Pacific Hwy \$8(through) Lane 1,2	1,776	53.6%	2.9	A	31
	Pacific Hwy SB (right and through)	1,698	96.4%	31.9	C	193
	Bridge St (left)	180	31.4%	49.1 ·	C	19
AM .	Pacific Highway NB (through) Lane 2,3	1,512	59.7%	4.5	A	42
	Pacific Highway NB (left and through)	763	58.0%	5,0	A	85
	Pacific Hwy S8(through) Lane 1,2	1,144 .	35,7%	2.1	A	15
	Pacific Hwy SB (right and through)	1,355	78.0%	11.5	A	65
	Bridge St (lo(1)	383	69.2%	60.4	£	49
PM	Pacific Highway NB (through) Lane 2,3	1,804	68.0%	6.7	A۰	36
	Pacific Highway N8 (left and through)	817	64.6%	7.2	A	119

Table 6.11: Scenario 4 - Pacific Highway/Bridge Street (Optimised Results)

[1] Level of Service (LOS) based on the RTA criteria for vehicle delay.

[2] Assume 6.0m per pcu,

Table 6.11 indicates that:

- During the Weekday AM and PM peak hour the Bridge Street leg of the intersection experiences delays and moderate queuing for the left turn only movement onto the Pacific Highway.
- The Pacific Highway experiences queuing during the AM peak hour (southbound) and PM peak hour (northbound) under the Scenario 5 intersection layout.

6.4.3 Ryde Road/West Street

Table 6.12 presents a summary of the future operation of the Ryde Road/West Street priority intersection during the identified network peak-hours.

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Peak	Approach	Demand Flows (pcv)	DOS	Average Delay (sec/pcu)	LOS [1]	Average Maximum Queue (m) [2]
	Ryde Rd EB (left and through) Lane	1,043	73,4%	10,3	Å	116
AM	Ryde Rd EB (through) Lane 2,3	2,287	72.7%	. 8.6	A	115
	West St (left) Lane 1,2	185	75.5%	96.6	F	34
РМ	Ryde Rd E3 (left and through) Lane	971	83.6%	26,4	В	183
	Ryde Rd E8 (through) Lane 2,3	2,228	· 86.0%	25.9	8	215
`	West St (left) Lane 1,2	522	88.8%	69.1	E	102

Table 6.12 indicates that:

During the Weekday AM and PM peak hour the West Street leg of the intersection experiences delays and moderate queuing for the left turn only movement onto Ryde Road.

During the Weekday PM peak hour the Ryde Road leg of the intersection experiences moderate delays and queuing for the eastbound approach to the intersection.

Total Intersection and Network Operation Results 6.5

Table 6.13 has been prepared to summarise the overall intersection operating performance for all modelled scenarios. The three parameters that LinSig provides for the intersections are:

- Practical Reserve Capacity (PRC) available capacity within the intersection, with a negative result indicating that the intersection is over capacity
- Total delay per hour combination of delay for all vehicles in hours
- Degree of Saturation (DoS) percentage of actual flow and saturation flow.

J\$11530 Pymble Business Park Transport Assessment of Access Improvements



Intersection	Peak	Parameter	Existing	Scenario 1	Scenario 2	Scenario 3	Scenario 4
Pacific Highway/Bridge Street	лм	PRC	10.7%	-4.9%	-15.0%	-32.4%	-7.1%
		Total delay	9.5	18.0	- 59.6	79.0	21.6
		Do\$	81.3%	94.4%	103.5%	119.2%	96.4%
	РМ	PRC ·	-0.5%	32.1%	-8.9%	-22.9%	15.4%
		Total delay	· 13.6	11.1	. 18.8	216.7	16.4
		DoS	90.5%	68.1%	82.7%	110.6%	78.0%
Pacific Highway/ Ryde Road/ Mona Vole Road	АМ	PRC	-18.3%	-27.6%	-27.6%	-28.3%	-24.0%
		Total delay	199.4	437.2	411.5	412.3	377.9
		DoS	106.4%	114.8%	114.8%	115.4%	111.6%
	РМ	FRC	-6.5%	-5.1%	-9.2%	-18.7%	-20.3%
		Total delay	98.4	97.9	132.7	207.0	154,2
		Do\$	95.8%	94.6%	98.2%	106.8%	108.2%
Rydø Road/ West Street -	АМ	PRC	-	• -	21.7%	27.5%	19,2%
		Total delay	-	-	15.8	11.2	15.7
		Dos	-	-	74.0%	70.6%	75.5%
	РМ	PRC	-		-2.3%	10.6%	1.4%
		Total delay	-	-	37.2	19.7	37.1
		DoS	-	-	92.1%	81.4%	88.8%
Total Network	лм	PRC	-18.3%	-27.6%	-27.6%	-32.4% -	-24.0%
		Total delay	208.9	455.2	486.9	502.5	415.4
		DoS	106.4%	114.8%	114.8%	119,2%	11.6%
	РМ	PRC	-6.5%	-5.1%	-9.2%	~22.9%	-20,3%
		Totol delay	112.0	109.0	188.6	443.4	207.7
		DoS	95.8%	94.6%	98.2%	110.6%	108.2%

Table 6.13: Summary of Overall Intersection Operation

6.6 Future Bus Stops

Included within the Project Brief was the requirement to identify suitable locations for future bus stops to improve accessibility to public transport services.

For eastbound services on Ryde Road, it is recommended that a bus stop could be located approximately 90 metres west of West Street, outside 25-31 Ryde Road. A bus stop at this location would avoid conflict with eastbound vehicles attempting to access either West Street or the Ryde Road off-ramp, be advantageous from a gradient perspective, and would ensure that the majority of businesses within the Park would be within a 400 metre walk of the bus stop.

For northbound services on the Pacific Highway, it is recommended that a bus stop could be located approximately 190 metres south of Ryde Road/Mona Vale Road, outside 880 Pacific Highway (Alto Mitsubishi). A bus stop at this location would avoid conflict with northbound vehicles attempting to access the Ryde Road on-ramp turn bay and would be advantageous from both a gradient and a vehicular sight distance perspective.

J\$11530 Pymble Business Pork Transport Assessment of Addess Improvements
Conclusions



7. Conclusions

The key outcomes of the assessment undertaken by GTA Consultants are as follows:

- i The total network under existing conditions is operating in excess or near to operational capacity during the Weekday AM and PM peak hours respectively.
- ii Scenario 1 with background growth and development traffic with no intersection works results in the modelled network exceeding capacity with significant delay and gueuing.
- iii Scenario 2 with the inclusion of the Pacific Highway pedestrian crossing at Bridge Street and West Street signalisation at Ryde Road results in minor additional delay for the total network.
- Scenario's impact of the Bridge Street right turn onto the Pacific Highway has a negative impact
 on the total network operation, though presents benefits for West Street traffic at Ryde Road.
- Scenario 4 provides additional Ryde Road left turn capacity on approach to the Pacific Highway
 with total network operation results similar to those presented in Scenario 2.
- vi Of the three improvement options prepared by Council, the signalisation of the Ryde Road/West Street would be an essential requirement to mitigate the traffic impact associated with any future growth in the Pymble Business Park.

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Appendix A

Appendix A

LinSig Modelling Results

JS13530 Pymble Business Park Transport Assessment of Access Improvements 14/03/11 Issue: A





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68.1% 1916 1423 - 53486 1916 1492:1352 1800 1800 1776 1698:1800 1800:1800 1558:1800 247 1744:1800 1744:1500 1800:1800 225 641 88 85 B ig 89 89 624 623 434 1226 293 328 ത 527 906 527 11 590 R 867 C1:5 C1:6 21:7 C1:8 Q:1 Q:2 C1:5-013-01 0 U 0.12 ្ពុដ្ឋ 0 01:4 C1:7-5 019 0 0 ซี 3 88 100 ສື 8 8.8 ü ï ٩ ЧÅ 쏚 ٩ş Å ş ٩X ΑŅ Ψþ Ψž Ξ Νŝν Ϋ́ ٩J ₹Ņ ₽Ă ΜĄ ΝţΑ Nŝa ΥŻΝ MA ž ₹ ₹N N A ٩N Ň N;A Ϋ́β đ. đ.N ş MA ЧN Ä ΝN ¥. Νă NG Niv N ŝ Ř NN Ň 0 t Ŷ 4 9 Þ ÷ ∋ Þ C Mona Vale Rd East Left Right Ahead Mona Vale Rd East U-Tum Right Ryde Rd West Right Ahead Lefi Pacific Hwy North Ahead Right Pacific Hwy South Ahead Left Padific Hwy South Afread Left Ryde Rd West Right U-Turn Pacific Hwy South Ahead Pacific Hivy South Ahead Pacific Hwy North Ahead Pacific Hwy South Ahead Pacific Hwy South Ahead Pacific Hwy North Ahead Padific Hwy North Ahead Pacific Hwy North Ahead 「「「「「「「」」」」」」 Pacific Hwy North Right Pacific Hwy North Right Pacific Hwy South Right Mona Vale Rd North Ryde Rd West Right Bridge St West Left Ryde Rd South Ryde Rd Exit Bridge St

E.Jz: Pactic Hwy Bridge St

1/3÷1/4

301 55 ES

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EE Ped Link: P1

5/2+5/1

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1/2+2/2

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11.13 108.99 Cycle Time (5): 97.61 Total Delay for Signalled Lanes (pouhr): Total Delay Over All Lanes(pcuHr): Total Delay for Signalled Lanes (pcuHr); 5.1 PRC for Signalled Lanes (%): PRC Over All Lanes (%): PRC for Signaled Lanes (%):

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Filer	T Show Exit Lanes	PCUIs and Capacity Values	Tree Columns - Collapse		Column Set 1; Use	Saye	-View Type • Tree View							
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E Network	いたのではない		16.	٢.	-	- 38	で の の の の の の の の の の の の の	prul 3at poun	1 pount	stocni	boul			
1			t		: · · · · ·		1 1 1	- 114.8%	411.5	1 2 2 2	•,			
1/241/1		Pacific Hwy North Ahead Left	U N¦A	NÌA	- 1110	907	1800:1800	796 113.9%	74.8	296.9	132.9			
27T		Pactic Hwy North Ahead	N'N N	N/A	011	877	1300	771 113.7%			96.5			
- 75		Pacific Hwy North Aread	AN U	N/A	111	35 25	1800	771 107.3%	46,4	202.0	68,6			e a starter
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4/2+4/1	-	Padife hwy North Rught Manadife od carrained		NIA	C1:2		1800	283 96.7%		134.7	16.8			
**************************************	_	irona vale ku bast leit Nught Ahead Moore unte ou erster trom or co	0	NIA	13- 13-		1744:1800	376 114,8%		319.2	45,4			
5/2+5/1		Provide Maler Kultast U-Turth Kught Domise Hum South Aligonal Lee		N(A	0.50.6		1744:1800	-	•••	357.5	30.8			2715)
5.3		רבנוא זאיץ שנונו הואסט נפו. Denfin burn כאילה לאיים		12			1500:1500			31.4	17.1			775
5/4		radic him South Alexan Paritic Himu South Alexan		H N		518	1800			39.1	17.1			19780
55		r detre 1979 Joods of Factor		τù.		208	1800	771 65.9%		38.6	16.6			
112+217	-	Fault Del Work Sound Frault.		N/A	1:4		1800	270 112.6%		329.7	33.8			
28	_	kyde ko west kight Ahead Left	N/N OHO	NA	C177	261	1558:1800	516 108.8%	32.9	211.0	42.9			4177
212-712		Ryde Rd West Right	U N/A	N/A	017	248	1698	230 107.6%	6 18.5	269.2	23.7			
1111/3 62		Ryde Rd West Right L-Turn	U N/A	N¦A	C1:7 C1:8	239	1693:1800	539 109.4%	6 36.2	- 221,2	40.7			17.3 17.3
		Ryde Rd Exit	AX U-	A/A	ł	291	3600	3600 7.8%	0,0	0.5	0.0			
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11/341/4		Pacific Hwy North Ahead	U N/A		13	877	1800	1556 56.4%	6 <u>1</u> .3	5,2	9.7		•	
30+30		Pacific Hwy North Ahead Right	V/N O+N		C2:1 C2:2	1690	1800:1800	1633 103.5%	51.0°	108,7	111.8			nyy Vali
		Pacific Hwy South Ahead Left	U N/A		C13	765	1776:1800	1250 59.5%	3 1.2	6.0	14.2			•••• <i>•</i> ;;
() () () ()		Pacific Hwy South Aread	U N/A		C2:3	Ş	1916		. 1.2	6.2	5.0			
RSTAR		Pacific Hwy South Ahead	U N/A		C3:3	308	1916	1259 61.7%	. 1.3	5.9	3.3			
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		Pacific Hwy South	- N/A	•	C2:6	0	. '	3600 0.0%	°.0	0'0	0.0			in an
			N/A -	۱	•	ſ	•	- 74.0%	5 15,8	ſ	t			
Υ/T Υ/T		West St North Left	U N/A		3:1 3:1	. 22	1730	161 44.8%	° 1.6	80.2	3.0			20
		West St North Left	N/A N/A	NţA	ö.	112	1730	161 69.7%	°. 3.0	0.79	5.3			oninte
21 21		Ryde Rd Exit Ahead	U NA		۱.	706	1730	1730 40.8%	5 Q.3	1.8	0.3			(1000)))))
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DRC For S	-27.6 Tot	20 Dolore Concentration		ŀ										Ņŗ Ţ
C2 PRC for Signaled Lanes (%):		rocal belay for Signalled Lanes (pourty): Total Delay for Signalled Lanes (pourty);	411,5U 59,59									•		ene Fran
C3 PRC for Signalied Lanes (%): PRC Over AH anes (%):	21.7 Tot	Total Delay for Signalled Lanes (pourt): Total Delay Occas All Location and	88				-		•					
	0.12	Induced Over Autanesiptum):	400.00 Lycle Time (s):	(5): 140	-									
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fe kil f Stream f Lane f Route	r 🗂 Show Exit Lanes	C Display Per Period C Display Per Cycle	Collapse	Customise		Column Set 1: Use	Use Save		G Tree View							
the second s			<u> </u>	Lne	Ble	Ed	11 -		Sat Flow	Faul Ded	al TDu	Di.	S MAD			
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— Network — J1: Pacific Hwy/Ryda Rd/Mona Xale Rd				N.S.				会議委会	の利用が	2:186	127					
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13		Padric riwy North Ahead Left		din .		-	-						3 22.5			
2	φ	Pacific Hwy North Ahead	-	u Nya		C1:1		555	1800	681 81.4%	% 7.9	51,0	21.4			
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C/T	Pa	Padfic Hwy North Right	~	N/N N	N/A	<u>C1:2</u>	~7	346	1800	476 72.7%						
	^D	Pacific Hwy North Right	₽	AN (N/A	C1:2		342	1800	476 71.9%						
4[Z+4]1	Ma	Mona Vale Rd East Left Right Ahead	٦ ٦	UHO N¦A	N/A	<u>0:5-</u>	, 11	279 174	1744:1800 2	264 93.2%						
	Wo	Mone Vale Rd East U-Turn Right		n/v	N/A	C1:5 C1:6	, N	250 174	1744:1800	256 97.5%						
	÷ d	Pacific Hwy South Ahead Left	<u>ک</u>	AN OH	NíA	ເ		534 1800	1800:1800	960 97,3%						
	Υ. Δ	Pacific Hwy South Ahead	n	i N¦A	N/Α	013	Ţ	649	1800	681 95.2%						
4 5	Υ. Φ	Pacific Hwy South Ahead	5	J NJA	N(A	с: С:3	Ţ	648	1600	681 95,1%						
55	rêd.	Pacific Hwy South Right	D	J N/A	N/A	C1:4	л,	445	1800	463 96.1%						
·	Ry	Ryde Rd West Right Ahead Left	'n	J+O N/A	NįA	C1:7 ~	Ψ.	606 155	1558:1800 (
1/3	Ry	Ryde Rd West Right	ກ	3 N/A	NĮA	C1:7	***	147	1698	194 75,8%						
7/4+7/5	Ry	Ryde Rd West Right LFTurn	,	N/A	ΝĮΑ	C1:7 C1:8	1,	547 169		583 53,5%						
6/2	Ry	Ryde Rd Exit	_	J N/A	N/A	1	.,	344	8 8 8 8 8							
· Æ Ped Link: Pl	Mo	Mona Vale Rd North	•	N/A	,	C1:9		ب	- 27257							
		Ryde Rd South	,	NIA	,	C1:9		11	- 27253							
E-12: Pacific Hwy/Bridge.st	経動の経営を開発院			N/A	•	1. A.					New Street	NAMES OF A	3			
		Pacific Hwy North Ahead	. 	N/A	N/A	5	, ω : :	613	1800 15	1556 39.4%		200 C	a A A			
1/2	ē	Pacific Hwy North Ahead	Ð	nţA (Чţ	5 8	ι'n	555	1800 15	1556 35.7%		3.7				
1/3+1/4	. Pai	Padilic Hwy: Norith Ahead Right	ň	HO N/A	ЧA	C2:1 C2:2		1331 1800	1800:1800 16	1610 82.7%		16,4				
3/2+3/1	Par	Pacific Hwy South Ahead Left		i N/A	ЧŅ	C2:3	w	825 1776		1249 66.0%		1-				
	Pai	Pacific Hwy South Ahead	_	AįN I	Ч'n	C2:3	ŝ	876	1916 IS	1323 66.0%		7.2				
	æd.	Pacific Hwy South Ahsad	*	N/A	ΝįΑ	C:3	U)	620	1916 13	1328 69.3%			4.9			
		Bridge St West Left	•	ain i	μţ	C3:4	(1)	383 1492	1492:1352 5	564 68.0%	6.a 5.a	ω,	0.8 0			
		Bridge St		NIA	1.	ß			- 49	49836 0.0%		0,0	0.0	•		
:		Pacht Hwy South	•	A/A	۰	C3:6	•	8	ਲ '	3600 0.0%		0.0	0.0			
dekolwest St		「「大学」の一般の記録の「「大学」」	, .	A/N					のないのない	₩1-25 ~						
,	ψe	West St North Left	ب	N/A	NA	ថី	•	161	1730	395 40.7%		23.6				
2/1	We	West St North Left	بہ	N/A	NfA	ខី	е л	364	1730 3	395 52,1%		97.3	18.3		•	
2/1	R	Ryde Rd Exit Ahead	بہ	N	NIA	•	10	1036	1730 17	1730 59.9%		2,6				
2/2	, Ryi	Ryde Rd Exit Ahead Ahead2		A/N I	NIA	•	. 12	1296	1870 - 16	1870 69.3%		3.1				
233	Ryn	Ryde Rd Exit Ahead	ر	NIA	N/A	ı	. 12	1297	1870 18	1870 69.4%		3.1				.Ψ
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ut menuation agricultures (%); C2 · PRC for Sonaled Lanes (%);	-9.2 IotalD 8.9 TotalD	Jotal Delay for Signalled Lanes (poutry); Tutal Delay for Sunaled Lanes (northy)	132.60 18 70													
		Total Delay for Signalized Lanes (north):	34.17													
	-9.2 To	Total Delay Over All Lanes(pourh):		Curle Time (c).	ch. 14D											

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			DWW		1	130.4	93,2	71.1	21.8	21,8	46,6	30.1	17.3	15.7	17.0	33.6	42.9	21.8	38.3	0.0	0,2	0.3		10.4	9.7	101.7	14.9	ZU.1	30.8	0.2 0.2	8.0	1	1.5	2.9	0.3	0,8	0.9	15.7	15,9			
			Dly 1			279.4	281.3	205.5	186,0	186.0	327.6	350,8	31.0	36.9	38.2	329,7	216.3	262.0	226.6	0.5	22.7	22.7	•	ນ. 4			2.1	α α α	410.6	11.1	0.0		0'16	107.0		2,6	2,6	7,9	6.7			
			TDly	502.5	412.3	71.2	69.0	48.2	14.5	14.9	39.4	25.6	, 5,9 ,	ы. 1	ហ ហ	27.8	33.7	17.0	34.0	0.0	0,1	1.0	79,0	1.4	1.3	40.4	0 0 -	א נ קיי	30 B	0.0	. 0.0	11.2	0.9	1.7	0.3	0.8	6.0	2.3	2.2			
			Ceg Sat	119.2%		813 112.8%	784 112.6%	784 107.6%	283 102.3%	283 102.3%		113.5%	72.4%	63.1%	, 6 6. 4°	270 112.6%	513 109.2%	218 106.7%	109.5%				1			-		%J'KC 1	***	•		• •						•				
adkj	Tree View	Flat View	at Flow Cap pcu/Hr pcu	1.50			1800 784		1800 283		800 375	800 232	800 949	1800 784	1300 784			1698 218	800 493	3600 3600	. 31371	- 31371						1916 1163		4	- 3600	, 1	1730 99	1730 99	1730 1730	1870 1670	1870 1870	1790 1471	1870 1630			
Li L'Yiew Type			5		•	3 1800:1800					•	3 1744:1800	7 1600:1800				0 1558:1800		0 1698:1300		ω		· . ·			9 1800:1800	0,1		1492		0	`	34	8	•							
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	*****	n	P te			Ÿ							U/A C						-	N/A	۳ ۱		•		Ì	N N N			~		1	•	N/A	U∤A	M∕A	N/A	N¦A	NłA	N/A			140
	Customise		Stm Ed	, ANW	N/A	N(A	N¦A	Nja	N/A	μļa	N/A	N/A	ми м	NA	ЫA	N/A	e N/e	ΨŅ	e/N	NA	NA	N/A	4 2	N/A N/A		ele Ne	e e e e e e e e e e e e e e e e e e e	e Nie	N/A	N/A	N/A	ΝA	NįA	Nja .	M/A	NļA	d∤N	∀¦N	N/A			C vrie Time (c):
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l can		C Display Per Cycle Expand	- 9			Pacific Hwy North Ahead Left	Pacific Hwy North Ahead	Pacific Hwy North Ahead	Pacific Hwy North Right	Pacific Hwy North Right	Mona Vale Rd East Left Right Alread	Mona Yaie Rd East U-Turn Right	Pactic Hwy South Ahead Left	Pacific Hwy South Ahead	Pactific Hwy South Alread	Pacific Hwy South Right	Ryde Rd West Right Ahead Left	Ryde Rd West Right	Ryde Rd West Right U-Turn	Ryde Rd Exit	mona vale ku Noruh		a strategy and state	Paulik Fawy Nordh Anead Dacfic Haw North Ahead	rauter they would repaid	rauku My World Albad Ngk Pacific Hwy South Ahead Left	Partific Hwy Sarah Ahead	Pacific Hwy South Ahead	Bridge St West Right Left	Bridge St	Pacific Hwy South		West St North Left	West St North Left	Ryde Rd Exit Aheed	Ryde Rd Exit Ahead Ahead2	Ryde Rd Exit Ahead	Ryde Rd West Ahisad Left	Ryde Rd West Ahead			local Delay For Signaled Lanes (pcuHr); Total Delay Over All Lanes(pcuHr); 51
_	Show Exit Lares	snave Levers	Item Desc	なるがするか とうたいかい	t 1	Pacit	Pacif	Paci	Paci	Paci	uaM :	Mon	Pad	- Paci	Paci	Paci	RYG	Ryd.	Ryd	Ϋ́́Α	L	ΥΛΟ Υ		Lau Dag		Paci Paci	Land Land	Pæd.	Brid	. Brid	. Pad		- We	We	Ryd	Ryd	Ryd	. Ryd	. Ryd			1003
	L. L. States	1 1	item		Mona Yak Ku																																				anes (%): -32,4	
	C Stream				1. Pacific HwyjRyde Roll 1. Pacific	1/271/1	- fr	۲. ۲. ۱	c)t	1/0 41-1411	7/277/J1 412-14/4	10	uperaju Eks	5 7 1	2/1 E/E	76-275	7/3	714.775	. c)/TT// 80	uya Er Ded Fiskr D1	E Ped Link: P?		arakarakan di tetan. M	1/2	1/3+1/4	3/2+3/1	3/3	3/4	· 5/2+5/1	E Ped Link: P۱	I Ped Link: P2	E 33: Ryde Rd/West St	1/1	1/2	2/1	2/2	2/3	3/1	3/2	PRC for Signalizd Lanes (%):	PRC for Signaled Lanes (%): ppC for Secondard Survey (%).	PRC Over All Lanes (%)
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Lanes (% Lanes (% Lanes (%	• •					•				•							:														•			d/Mona	作品を設置	llem	10 10 10 10 10 10 10 10 10 10 10 10 10 1			ж
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PRC for PRC for PRC for	<u>со</u>	~		^ i	***4	E 33: Ryde Rd/West St	🗉 Ped Link: P2	The Ped Link: PI	5(2+5(1	**	m	3/2+3/1	1/3+1/4	~		C 12: Radie Hwy/Bridge/St	函 Ped Link: P2	🕀 Ped Link: P1	~	7/4+7/5	m	7/2+7/1	10		m	5/2÷5/1	eletele		4/10	0		m	1/2+1/1	🖂 11: Pacific Hwy/RydeRd/Mona ValeRd			Contraction State	G All C Stream C Lane C Route		Network Results View
្ពាលខ	233	2/2	52	<u>4</u>	11	а Я Ш	£ Pe	એ દિ	512	Э.Ч	<u>3</u> 7	35	i.	ŭ.			ā. 图	ă. E	8/2	1	ΩŻ2	22	20	5 1	23		4 I	ν F	¢ (r F	1/4	្រុ	2	e H	- Network			, All	Filter	Netwo

Appendix B

Appendix B

SIDRA INTERSECTION Modelling Result

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J\$11530 Pymble Business Park Transport Assessment of Access Improvements 14/03/11 Issue: A

JS11530 - Pymble Business Park Int of Ryde Rd & West St 2011 Weekday AM Giveway / Yield (Two-Way)

Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back of Vehicles	f Queuo Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	Sec		veh	m	· · · · · · · · · · · · · · · · · · ·	per veh	km/
South: R	yae ka	•••	이 아이들이 같		4.0821 <u>4.19</u> 26			a Chegoria	1999 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -	i i i i i i i	
1	L. '	348	2.1	0.539	8.8	LOS A	0.0	0.0	0.00	. 1.04	53.1
2	T	2686	5.5	0.539	0.0	LOSA	0.0	0.0	0.00	0.00	70,
pproach	1	3035	5.1	0.539	1.0	LOSA	0.0	0,0	0.00	0.12	67.
Vest: We	ost Stre	ot 👘		Sal Store		2331074340V				N N 18 P. I	
10	L	143	2.9	0,757	55.9	LOS D	5.0	30,9	0.97 [.]	1.25	23.
pproach	1	143	2.9	0.756	55.9	LOS D	5.0	30.9	0.97	1.25	23.

LOS (Aver. Int. Delay); NA. The average intersection delay is not a good LOS measure for two-way sign control due to zero delays associated with major road movements.

Level of Service (Worst Movement): LOS D. LOS Method for individual vehicle movements: Delay (RTA NSW). Approach LOS values are based on the worst delay for any vehicle movement.

Processed: Tuesday, 1 March 2011 3:08:34 PM SIORA INTERSECTION 4.0.19.1104 Project: P:VS11000 - JS11990\JS11530 Pymble Business Park Traffic Assessment\Sidra & Scates\SIDRA\2011 Base\110225sid_JS11530 RydeRd_WestSt.sip 8000056, GTA CONSULTANTS, FLOATING

SIDRA -----

JS11530 - Pymble Business Park Int of Ryde Rd & West St 2011 Weekday PM Giveway / Yield (Two-Way)

Mov ID		Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	. Average Speed km/h
South: R	yde Rd (W)				an a					
1	L	82	0.0	0.523	8.7	LOS A	0.0	0.0	0.00	1.40	53.1
2	٦	2948	1,6	0.524	0.0	LOSA	0.0	0.0	0.00	0.00	70.0
Approact	1	3031	1.6	0.524	0.2	LOS A	0,0	·0.0	0.00	0.04	69.5
Nost: Wo	est Stree	t				•	鐈				``
10	L, .	405	0.5	1.206	243.9	LOS F	57.2	316.0	1.00	4.05	8.0
Approact		405	0,5	1.206	243.9	LOS F	57.2	316.0	1.00	4.05	8.0

LOS (Aver. Int. Delay): NA. The average intersection delay is not a good LOS measure for two-way sign control due to zero delays asso-ciated with major road movements.

Level of Service (Worst Movement): LOS F. LOS Method for individual vehicle movements: Delay (RTA NSW). Approach LOS values are based on the worst delay for any vehicle movement.

Processed: Tuosday, 1 March 2011 3:19:09 PM SIDRA INTERSECTION 4.0.19.1104 Project: P:\JS11000 - JS11990\JS11530 Pymble Business Park Traffic Assessment\Sidra & Scates\SIDRA\2011 Base\110225sid-JS11530 RydoRd_WestSt.sip 8000056, GTA CONSULTANTS, FLOATING

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-SIDRA 40 INTERSECTION

JS11530 - Pymble Business Park Int of Ryde Rd & West St 2021 Weekday PM - Scenario 2 Giveway / Yield (Two-Way)

Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delav	Level of Service	95% Back c Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Averag Speed
		veh/h	%	v/c	Sec	0011100	veh	m		per veh	km/
South: R	lyde Ro	I (W)	16 - C		n en						
1	L	98	1.1	0.569	8.8	LOSA	0.0.	0.0	0.00	1.39	53.
2	Т	3184	1.6	0.568	0.0	LOS A	0.0	0.0	0.00	0.00	70.
Approac	h	3282	1.6	0.568	0.3	LOSA	0.0	0.0	0.00	0.04	69.
Vest: W	est Stre	əət 👘	4.7								$[[A, a^{(i)}]_{i \in \mathbb{N}}]$
10	L	546	0.8	1.865	821,1	LOSF	169.9	941.6	1.00	7.20	2.0
\pproac	h	546	0.8	1.865	821.1	LOS F	169.9	941.6	1.00	7,20	2.

LOS (Aver. Int. Delay): NA. The average intersection delay is not a good LOS measure for two-way sign control due to zero delays asso-ciated with major road movements.

Level of Service (Worst Movement); LOS F. LOS Method for individual vehicle movements: Delay (RTA NSW). Approach LOS values are based on the worst delay for any vehicle movement.

Processed: Wednesday, 2 March 2011 3:44:43 PM SIDRA INTERSECTION 4.0.19.1104 Project: P:\JS11000 - JS11990\JS11530 Pymble Business Park Traffic Assessment\Sidra & Scates\SIDRA\2011 Base\110225sid-JS11530 RydeRd_WestSt.sip 8000056, GTA CONSULTANTS, FLOATING

140-11 ANK SIDRA ~ ` INTERSECTION

Site: 2021 AM_Scenario 2

JS11530 - Pymble Business Park Int of Ryde Rd & West St 2021 Weekday AM - Scenario 2 Giveway / Yield (Two-Way)

	Demand	HV	Deg. Sain	Average Delay	Level of Service	95% Back o	Distance	Prop. Queued	Effective Stop Rate per veh-	Average Speed km/h
Mov ID Turn	Flow									
South: Ryde Ro	voh/h (W)	%	V/C	Sec		veh	m		TEL VEIL	ATTR
1 L	396	2,1	0.585	8.8	LOSA	0.0	0.0	0.00	1.03	53.1
2 Т	2901	5.5	0.586	0.0	LOSA	0.0	0.0	0.00	0.00	70.0
Approach	3297 -	5.1	0.586	1.1	LOSA	0.0	0.0	0.00	0.12	67.7
West: West Stre	et	· · ·	1			清凉小	2 () 2 ()			
10 L	189	2.8	1.247	301.7	LOSF	32.1	197.7	1.00	2.96	6.6
Approach	189	2.8	1.245	301.7	LOSF	32.1	197.7	1.00	2.96	6.6

LOS (Aver. Int. Delay); NA. The average intersection delay is not a good LOS measure for two-way sign control due to zero delays associated with major road movements.

Level of Service (Worst Movement): LOS F. LOS Method for individual vehiclo movements: Delay (RTA NSW). Approach LOS values are based on the worst delay for any vehicle movement.

Processed: Wednesday, 2 March 2011 3:37:53 PM SIDRA INTERSECTION 4.0.19.1104 Project: P:\JS11000 - JS11990\JS11530 Pymble Business Park Traffic Assessment\Sidra & Scates\SIDRA\2011 Base\110225sld-JS11530 RydeRd_WestSt.sip 8000056, GTA CONSULTANTS, FLOATING SIDRA



Appendix C

Appendix C

Intersection Improvement Options

14/03/11

JS11530 Pymble Business Park Transport Assessment of Access Improvements

14/03/11 Issue; A



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